

December 19, 2022

Board of Commissioners of Public Utilities  
Prince Charles Building  
120 Torbay Road, P.O. Box 21040  
St. John's, NL A1A 5B2

Attention: Cheryl Blundon  
Director of Corporate Services & Board Secretary

**Re: *Reliability and Resource Adequacy Study Review – Updated Project Schedule for L'Anse au Diable Grounding Station and Wave Study for Dowden's Point Grounding Station***

In its correspondence dated July 25, 2022,<sup>1</sup> Newfoundland and Labrador Hydro ("Hydro") advised the Board of Commissioners of Public Utilities ("Board") that it would provide an updated schedule for the completion of work at the grounding site located in L'Anse au Diable. In the same correspondence, Hydro also advised the Board that as part of the Design Review for the Dowden's Point Grounding Station, a wave study would be completed by the fourth quarter 2022.

A summary of the proposed project schedule is provided in Attachment 1 and includes three appendices:

- 1) A plan for project execution in 2023 (Appendix A),
- 2) Major project milestones (Appendix B); and
- 3) Level of effort (Appendix C).

The results of the Dowden's Point Grounding Site wave study ("Wave Study"), completed by Tiller Engineering Inc. ("TEI"), are provided in Attachment 2. The scope of the Wave Study was to evaluate this breakwater's original design parameters. TEI concluded that based on the documentation provided, the design life of the breakwater was 100-year minimum and was designed to withstand the expected worst-case conditions. In its conclusion, TEI also noted that the risk of wave damage to a rubble mound breakwater is considered less than the similar breakwater in L'Anse au Diable given the sheltered conditions of Dowden's Point in Conception Bay.

Further, TEI concluded that the Dowden's Point breakwater was based on a wave height with a 200-year return and appears it has been factored to account for other height parameters and sea level rise of 1.0 metres or storm surge. TEI considered this a valid design. TEI noted that in the light of the importance of the breakwater in protecting the electrode site, and considering global climate trends, Hydro could consider use of a higher wave factor if there was a desire to increase the long-term reliability for the breakwater structure at Dowden's Point. TEI recommended that Hydro conduct continual inspections and monitoring of the breakwater to validate the current design and construction to determine if a higher wave factor is required. Hydro has reviewed and accepted these recommendations.

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<sup>1</sup> "*Reliability and Resource Adequacy Study Review – Design Review for L'Anse au Diable and Dowden's Point Grounding Stations,*" Newfoundland and Labrador Hydro, July 25, 2022.

Should you have any questions, please contact the undersigned.

Yours truly,

**NEWFOUNDLAND AND LABRADOR HYDRO**



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Shirley A. Walsh  
Senior Legal Counsel, Regulatory  
SAW/sk

Encl.

**Board of Commissioners of Public Utilities**

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**Newfoundland Power Inc.**

Dominic J. Foley  
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Regulatory Email

# Attachment 1

## Breakwater Rehabilitation

L'Anse au Diable

Project Schedule Summary



# Breakwater Rehabilitation

L'Anse au Diable

Project Schedule Summary



## **Project Schedule Summary**

The proposed project schedule is attached as Appendix A and provides a plan for project execution in 2023. The plan is contingent on this project starting in January 2023 to allow adequate time for design, tendering, and construction.

Engineering design by an external engineering consultant is planned to start in late February 2023 with issue for tender design drawings and technical specification complete by the end of April 2023.

The construction is to be publicly tendered in early May 2023, with contract award in early June 2023.

Key pre-construction activities include:

- The approval of the armour stone source;
- DFO approval of the fish habitat compensation design change; and
- Review and approval of the contractor’s safety plan.

Construction site work is to start soon after contractor mobilization to site at the end of June 2023, with anticipated construction completion mid-September 2023. Contractor demobilization is planned for early October 2023, after successful final inspection and site clean up.

Closeout documentation shall include the contractor’s as-built drawings and redline markup of the design drawings, which are to be submitted at the end of September 2023 for owner review and acceptance. Standard project closeout activities will then follow, with project completion scheduled for early November.

This will be a challenging project due to construction access restraints and associated risks working near water. Therefore, project construction methodology, which may including rock placement from a barge or vessel, will need to be carefully developed to ensure safe project execution. Collaborative discussions and planning with the construction contractor will be required to lessen potential impacts to the baseline schedule(Appendix A).

Documents outlining major project milestones and level of effort are attached as Appendix B and C, respectively.

# Attachment 1, Appendix A

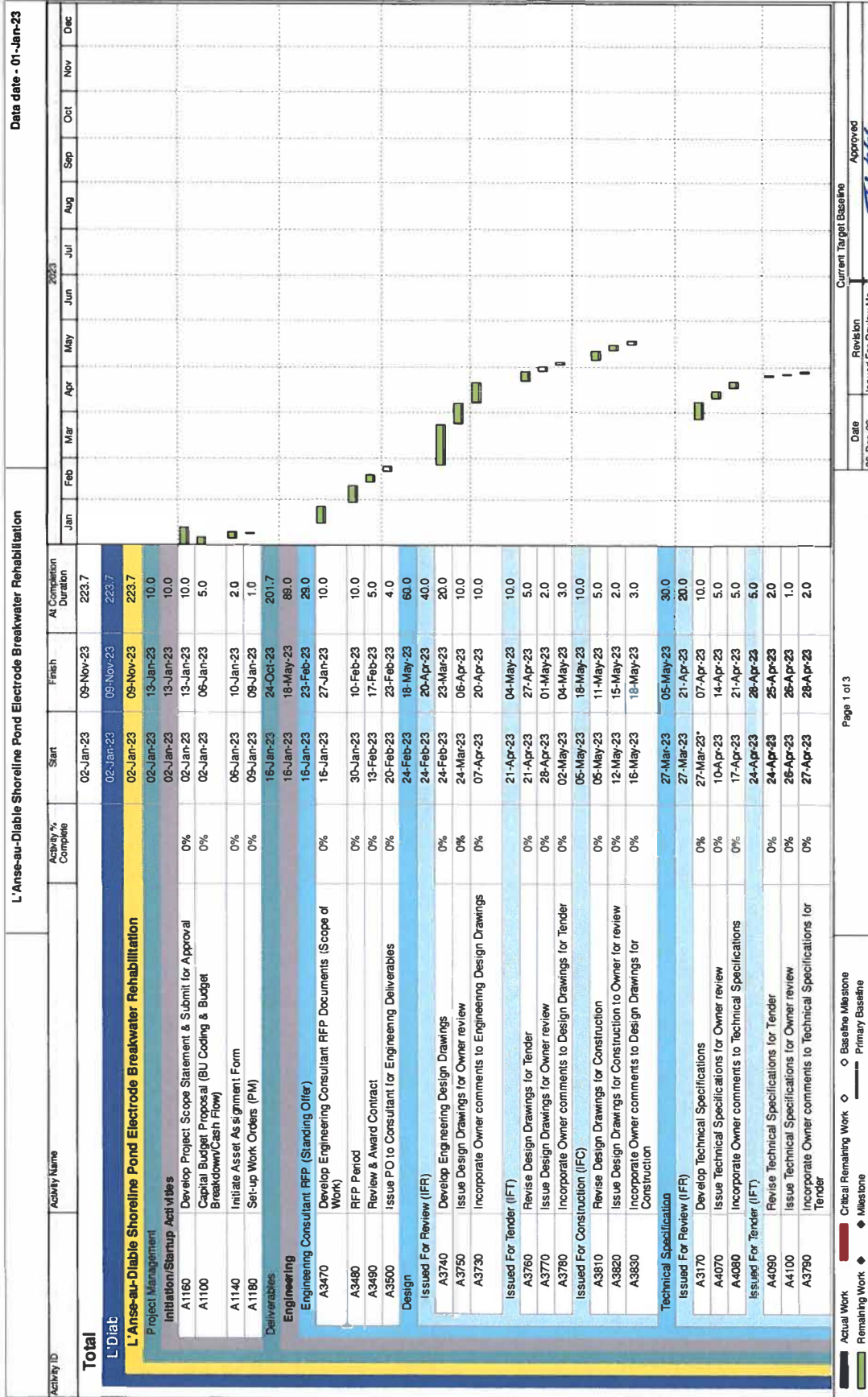
## Shoreline Pond Electrode Breakwater Rehabilitation

L'Anse au Diable

Baseline Schedule



Breakwater Rehabilitation - L'Anse au Diable - Project Schedule Summary, Appendix A



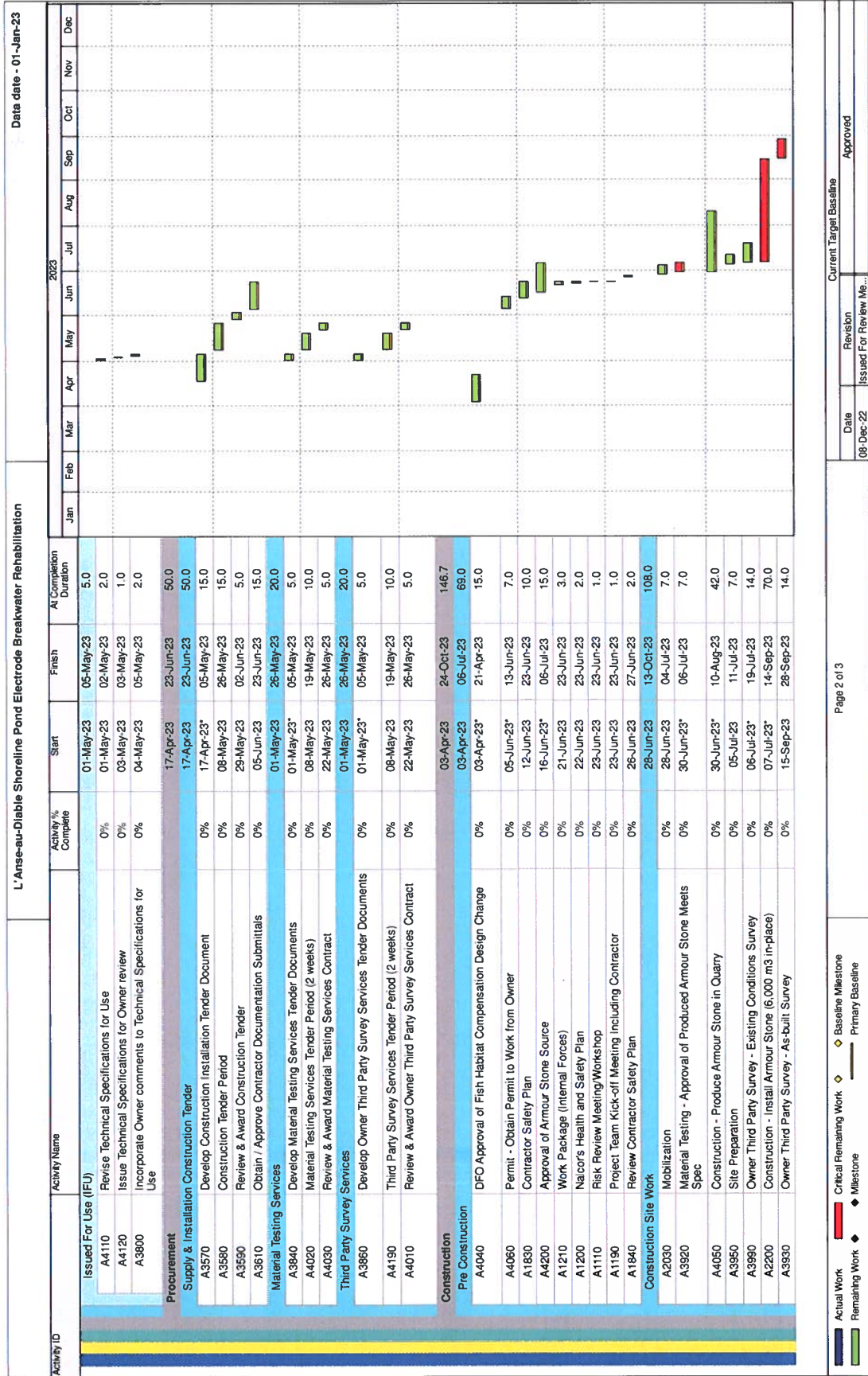
■ Actual Work  
■ Remaining Work  
○ Critical Remaining Work  
○ Milestone  
○ Baseline Milestone  
○ Primary Baseline

06 Dec-22

Revision Issued For Review Me

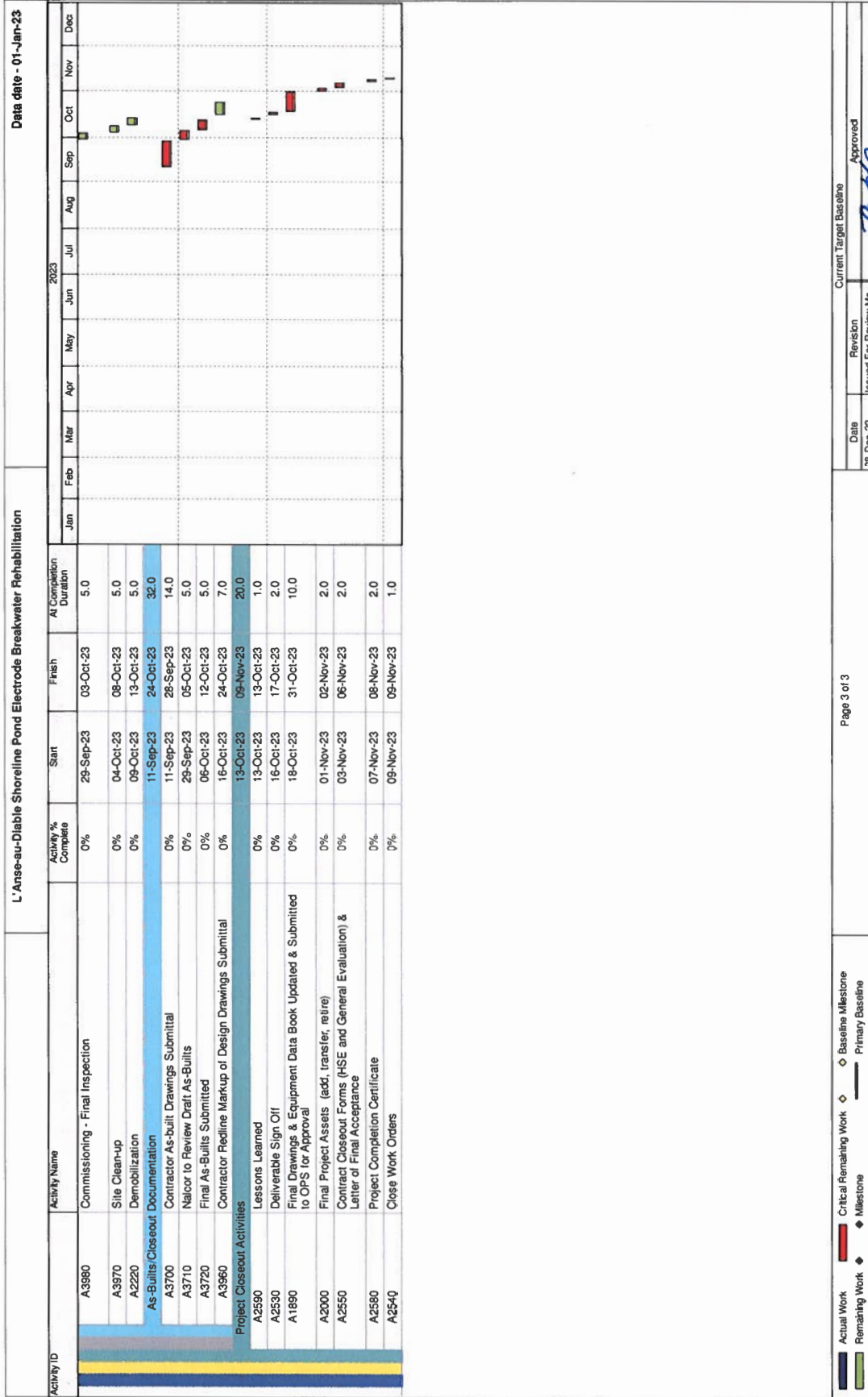
Current Target Baseline Approved

Breakwater Rehabilitation - L'Anse au Diable - Project Schedule Summary, Appendix A





Breakwater Rehabilitation - L'Anse au Diable - Project Schedule Summary, Appendix A



# Attachment 1, Appendix B

## Shoreline Pond Electrode Breakwater Rehabilitation

L'Anse au Diable

Major Project Milestones



Breakwater Rehabilitation - L'Anse au Diable - Project Schedule Summary, Appendix B

MAJOR PROJECT MILESTONES		L'Anse-au-Diable Shoreline Pond Electrode Breakwater Rehabilitation												
Activity ID	Activity Name	Start	Finish	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov
<b>Total</b>														
<b>L'Diab L'Anse-au-Diable Shoreline Pond Electrode Breakwater Rehabilitation</b>														
<b>Engineering &amp; PM Milestones</b>														
A1240	Project Approved	02-Jan-23	09-Nov-23											
A1220	Scope Statement Signed Off	02-Jan-23	09-Nov-23											
A4170	Engineering Design Drawings Issued For Owner Review	02-Jan-23	13-Jan-23											
A4160	Engineering Specifications Issued For Tender	02-Jan-23	06-Apr-23											
A4180	Engineering Specifications Issued For Owner Review	02-Jan-23	28-Apr-23											
A4150	Engineering Design Drawings Issued For Tender	02-Jan-23	04-May-23											
A2300	Risk Review Completed	02-Jan-23	23-Jun-23											
A2450	Quarry Approved for Armour Stone Supply	02-Jan-23	06-Jul-23											
A1250	Project Closed Out	02-Jan-23	09-Nov-23											
<b>Procurement Milestones</b>														
A2390	RFP Issued for Engineering Services	30-Jan-23	02-Jun-23											
A2340	PO Issued for Engineering Services	30-Jan-23	23-Feb-23											
A3880	Contract Tender Issued - Supply & Install of Breakwater Rehab.	08-May-23												
A4130	Contract Award - Owner Third Party Survey Services		26-May-23											
A4140	Contract Award - Material Testing Service Contract		26-May-23											
A3890	Contract Tender Awarded - Supply & Install of Breakwater Rehab		02-Jun-23											
<b>Construction Milestones</b>														
A2460	Approval of produced armour stone from quarry meets spec	06-Jul-23	03-Oct-23											
A2350	Construction Started	07-Jul-23	06-Jul-23											
A2360	Construction Finished		14-Sep-23											
A2330	Released for Service		03-Oct-23											

Date: 08-Dec-22  
 Revision: PSES Schedule Temp...  
 Approved: *[Signature]*

◆ Milestone  
 ◇ Baseline Milestone

# Attachment 1, Appendix C

## Shoreline Pond Electrode Breakwater Rehabilitation

L'Anse au Diable

Level of Effort





# Attachment 2

## Design Check Report

Dowden's Point Breakwater



# **Dowden's Point Breakwater**

## **Design Check**

### **Report**

**Prepared by:**

Tiller Engineering Inc.  
PO Box 403  
50 Hamlyn Rd. Plaza  
St. John's, NL,  
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<b>Issue and Date:</b>	<b>Status:</b>	<b>Project #:</b>	<b>Issued By:</b>	<b>Checked By:</b>	<b>Approved By:</b>
Dec 11, 2022	R1.2 Reissued	2022-114	RT	RT	RT
Nov 18, 2022	R1.1-Reissued	2022-114	RT	RT	RT
Sept. 16, 2022	R0-Issued for Review	2022-114	RT	JS	RT

## **TABLE OF CONTENTS**

<b>1.0 INTRODUCTION .....</b>	<b>3</b>
<b>2.0 PURPOSE .....</b>	<b>3</b>
<b>3.0 ABBREVIATIONS .....</b>	<b>3</b>
<b>4.0 REFERENCES .....</b>	<b>4</b>
<b>5.0 ORIGINAL DESIGN CRITERIA .....</b>	<b>5</b>
5.1 WIND SPEED .....	5
5.2 WATER CURRENT .....	5
5.3 WATER LEVELS.....	5
5.4 WAVE HEIGHT.....	5
5.5 DESIGN LIFE.....	6
5.6 BREAKWATER MATERIAL PROPERTIES.....	6
5.7 BREAKWATER DESIGN LOADS SUMMARY.....	7
5.8 BREAKWATER CREST HEIGHT .....	7
<b>6.0 CONSTRUCTION .....</b>	<b>8</b>
6.1 CONSTRUCTION METHODOLOGY.....	8
6.2 BREAKWATER CONDITION .....	8
6.3 AS-BUILT ELEVATIONS .....	9
<b>7.0 CONCLUSIONS AND RECOMMENDATIONS.....</b>	<b>10</b>

### **List of Tables**

- Table 1 – Table of Abbreviations
- Table 2 - Maximum Wind Speeds
- Table 3 - Significant Wave Heights
- Table 4 – Design Loads Summary
- Table-5 – Breakwater Crest Elevations

### **List of Figures**

- Figure 1 - 2010 Preliminary Breakwater Design
- Figure 2 – 2015 Issued for Construction Breakwater Design
- Figure 3 – 2016 As-Built Breakwater Survey

### **List of Appendices**

- Appendix A – Design Drawings



## 1.0 Introduction

Dowden's Point has been selected as the most suitable site for the island converter station electrode. The site is located on the south shore of Conception Bay, between Seal Cove Pond and Lance Cove Pond. The Dowden's Point breakwater was constructed in 2016 as a protection for shoreline pond electrodes as part of the Lower Churchill Project. The purpose of the breakwater was to protect the electrodes from wave and wind forces which otherwise may damage the electrodes.

## 2.0 Purpose

Tiller Engineering Inc. (TEI) was engaged by the client to review the project drawings, specifications and related construction documentation and to provide an independent opinion on the basis of design and construction processes associated with the breakwater.

TEI reviewed the documentation and the project drawings and specifications with respect to industry guidelines.

Industry Standards, Guidelines, and data used by TEI in this review included:

- i) National Building Code of Canada, 2015
- ii) Shore Protection Manual (US Army Corps of Engineers, 1984)

## 3.0 Abbreviations

The following abbreviations are used throughout the report:

<b>Abbreviations</b>	
TEI	Tiller Engineering Inc.
LCP	Lower Churchill Project
SPM	Shore Protection Manual
MSC	Meteorological Service Of Canada
m	Meters

**Table 1 – Abbreviations**

## 4.0 References

- [1] MFA-SN-CD-6300-CV-DC-0001-01 - Shoreline Pond Electrodes Civil/Marine Design Criteria
  
- [2] Wave Climate and Extremes at Dowden's Point, Conception Bay, May 2012, 505573-8600-41ER-0001 Rev. 00
  
- [3] Issued for Construction and As Built Project Drawings
  
- [4] Electrode Sites Breakwater Installation Technical Specification ILK-SN-CD-8600-CV-TS-0001-01
  
- [5] Shore Protection Manual – Department of the Army US Corps of Engineers 1984

## 5.0 Original Design Criteria

The breakwater design is intended to withstand the expected worst case site conditions, including wave action, tidal effects, pack ice and freezing inside the shoreline pond. The area of Dowden's Point has a maritime climate that is somewhat more temperate compared to other parts of the Avalon Peninsula. The average daily temperature is reported as 6.1 C and the total annual precipitation at 1127.2mm on average.

### 5.1 Wind Speed

The design criteria provided for the breakwater design shows that West and South West winds prevail and outlines monthly maximum wind speeds along with direction and average speed which can be seen below in Table 2.

Month	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec.	Year
Maximum Hourly Speed (km/h)	120	137	121	93	101	74	67	84	97	103	105	97	137
Maximum Gust Speed (km/h)	167	193	193	159	146	108	107	113	153	153	161	153	193
Direction of Gust	SW	SW	NW	SW	N	NE	SE	SW	S	NW	SW	S	S to SW

**Table 2 - Wind Speeds**

### 5.2 Water Current

The currents along the coast are reported to be 1 knot in the east-west direction.

### 5.3 Water Levels

The following water levels for Dowden's Point are provided:

Low Low Water = 0.2 m  
 Lowest Low Water = 0.0 m  
 High High Water = 1.1 m  
 Highest High Water = 1.3 m

### 5.4 Wave Height

The following Significant wave height data is provided in the Design Criteria Document:

Return Period (Years)	Extreme Wave Height, Hs (m)					
	Location					
	1	2	3	4	5	6
1	2.5 ± 0.1	3.1 ± 0.1	2.7 ± 0.1	2.8 ± 0.1	2.6 ± 0.1	2.5 ± 0.1
2	2.9 ± 0.1	3.5 ± 0.1	3.1 ± 0.1	3.1 ± 0.1	2.9 ± 0.1	2.7 ± 0.1
5	3.2 ± 0.1	3.9 ± 0.2	3.4 ± 0.1	3.4 ± 0.1	3.2 ± 0.1	3.0 ± 0.1
10	3.3 ± 0.2	4.1 ± 0.2	3.6 ± 0.2	3.6 ± 0.2	3.4 ± 0.2	3.2 ± 0.2
25	3.5 ± 0.2	4.4 ± 0.2	3.8 ± 0.2	3.9 ± 0.2	3.6 ± 0.2	3.4 ± 0.2
50	3.7 ± 0.2	4.6 ± 0.3	4.0 ± 0.2	4.0 ± 0.2	3.8 ± 0.2	3.5 ± 0.2
100	3.8 ± 0.3	4.7 ± 0.3	4.1 ± 0.3	4.2 ± 0.3	4.0 ± 0.3	3.7 ± 0.3
200	4.0 ± 0.3	4.9 ± 0.3	4.4 ± 0.3	4.3 ± 0.3	4.1 ± 0.3	3.8 ± 0.3

**Table 3 - Significant Wave Heights**

From this table, it appears that the Hs (significant wave height) of 4.9m with a return period of 200 years was selected as the Design Wave for the breakwater.

## 5.5 Design Life

The design life of the breakwater is reported to be 100 years minimum.

## 5.6 Breakwater Material Properties

The following material properties are reported for the breakwater materials:

Rock fill shall be assumed to have the following properties:

- dry unit weight of 1900 kg/m<sup>3</sup>
- submerged unit weight of 1100 kg/m<sup>3</sup>

Armor stone shall be assumed to have the following properties:

- dry unit weight of 2700 kg/m<sup>3</sup>
- submerged unit weight of 1900 kg/m<sup>3</sup>
- 35% in place porosity

Core Material

- The gradation of the core material shall provide sufficient permeability to maintain flushing of the shoreline pond.

## 5.7 Breakwater Design Loads Summary

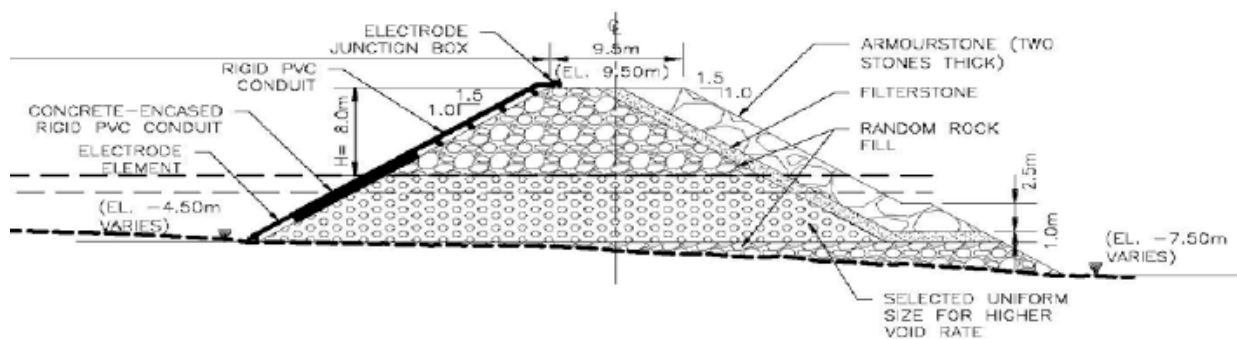
The following table summarizes original design values used for the breakwater:

Load	Basis
Wind Loads	Based on maximum hourly wind speed of 137 km/hr.
Wave Loads	Based on Design wave of 4.9 m
Current Loads	Based on 2 knots

**Table 4 – Design Loads Summary**

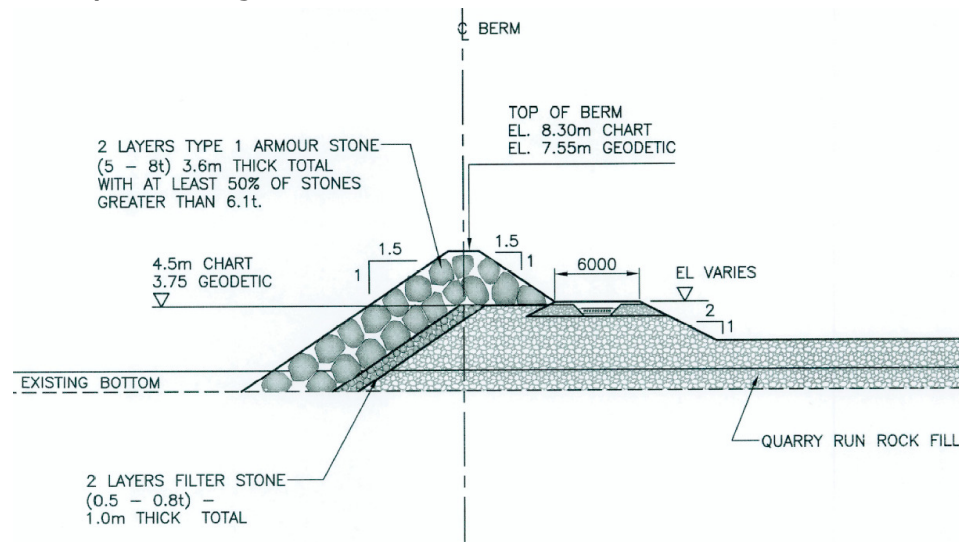
## 5.8 Breakwater Crest Height

2010 preliminary Dowden's Point Breakwater Designs detail a crest height of 9.50m. It is assumed that this is referenced to chart datum which would equate to 8.75m geodetic. A 2010 cross section is shown below:



**Figure 1 – 2010 Preliminary Breakwater Design**

Based on the 2012 Design Criteria, the 2015 Issued for Construction Design defines a 7.55m (geodetic) crest height as can be seen below:



**Figure 2 – 2015 Issued for Construction Breakwater Design**

## 6.0 Construction

### 6.1 Construction Methodology

Construction of a breakwater includes excavation of the sea bed, laying of the core, filter and armor stones. In the case of Dowden's Point the same processes were undertaken with the additional construction of electrode wells to facilitate the Lower Churchill Project (LCP). Core stones are first placed on the seabed in sections and filter and armor stones are sequentially added to protect the core stones from wave action. This procedure continues until the breakwater is complete. Crest width is an important factor to consider as heavy equipment is needed to lay armor stones and the crest must be wide enough for equipment to safely reach all points of the breakwater. Core, filter, armor stone sizes and width layers are all determined through design and the construction requires the breakwater to be built as close as possible to the design to retain the structural resistance against wave action.

### 6.2 Breakwater Condition

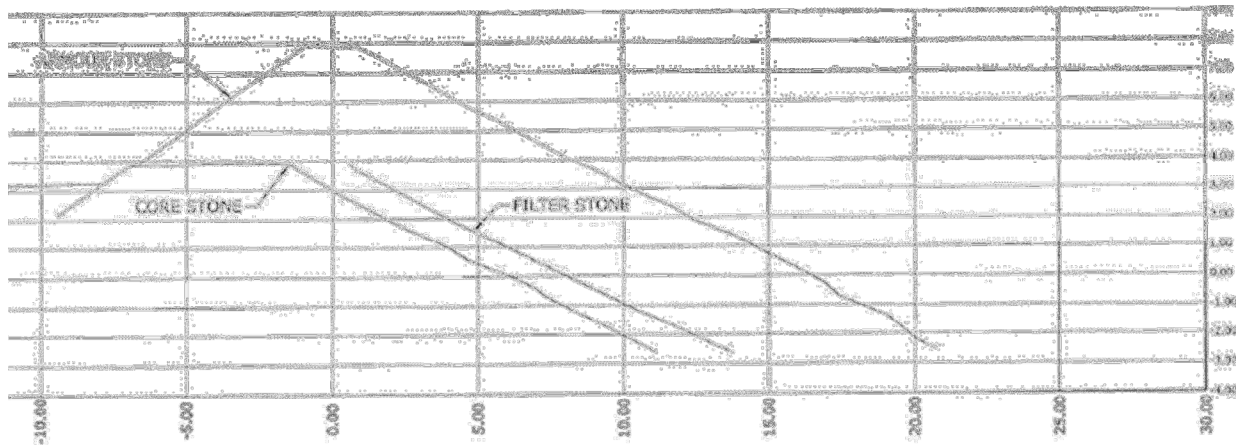
In June 2022, TEI performed a visual inspection of the breakwater. It appears to be in very good condition. The armour stones are well graded with a range of sizes providing a tightly stitched structure with very few occurrences of sizable voids between stones. The slopes are consistent along all faces as are the top elevations and alignments of the toes over the total length of the breakwater. The east and west sides of the breakwater extend well inland over the beach intercepting higher ground. Although the breakwater appears to be in very good condition and unaffected by any wave action to date, there is a local anomaly located at the center of the inside of the north side of the

breakwater at the toe of the slope. Several large (2-3 tonne) armour stones are displaced, previously buried filter fabric is exposed and smaller filter stone is displaced and scattered from its originally constructed location. It does not appear to have been caused by overtopping of the breakwater due to its very localized nature.

### 6.3 As-Built Elevations

As-Built project documents include a site overview as well as cross sections at various points along the breakwater. Elevations on the design drawings include height for core, filter, and armor stones; as-built elevations are to be as close as possible to these elevations as these elevations are the determined heights for the breakwater to resist wave action.

As-built surveys performed and verified after construction show a consistent crest elevation over the length of the structure of approximately 7.8m geodetic. A sample of the as-built survey results is shown below:



**Figure 3 – 2016 As-Built Breakwater Survey**

Table 5 below summarizes the design and as-built crest elevations:

<b>Breakwater Crest Elevations (Geodetic)</b>		
Cross Section Station	Design (m)	Actual (m)
0+050	7.55	7.8
0+075	7.55	7.8
0+125	7.55	7.8
0+175	7.55	7.8
0+225	7.55	7.8

**Table 5 – Breakwater Crest Elevations**

## 7.0 Conclusions and Recommendations

TEI performed an inspection of Dowden's Point breakwater in June 2022, and the breakwater structure, approximately six (6) years after construction is in very good condition.

Based on the documentation provided, the design life of the breakwater was 100-year minimum and the breakwater for the site was designed to withstand the expected worst-case conditions.

The Design Wave for the breakwater was taken as the 1:200-year Significant Wave Height ( $H_s$ ). It is stated that a sea level rise of 1.0m will be included in the Design and no discussion of storm surge water levels is included.

Reference [5] Shore Protection Manual – Department of the Army US Corps of Engineers 1984 provide s the following discussion of Significant Wave vs. Design Wave

The wave height usually derived from statistical analysis of synoptic weather charts or other historical data to represent wave conditions in an extreme event is the significant height  $H_s$ .  $H_s$  may be further defined in approximate relation to other height parameters of the statistical wave height distribution in deep water:

- $H_{1/3}$  or  $H_s$  = average of highest 1/3 of all waves
- $H_{10} = 1.27 H_s$  = average of highest 10 percent of all waves
- $H_5 = 1.37 H_s$  = average of highest 5 percent of all waves
- $H_1 = 1.67 H_s$  = average of highest 1 percent of all waves

Damage to rubble-mound structures is usually progressive, and an extended period of destructive wave action is required before a structure ceases to provide protection. It is therefore necessary in selecting a design wave to consider both frequency of occurrence of damaging waves and economics of construction, protection, and maintenance. If there is an increased frequency of waves in excess of the 1:200 year design height,  $H_s$ , the resulting effect could be progressive annual damage if left unaddressed. Here, a higher design wave of  $H_{10}$  or  $H_5$  may be advisable. Selection of a design height between  $H_s$  and  $H_5$  is based on the following factors:

- Degree of structural damage tolerable and associated maintenance and repair costs (risk analysis and life-cycle costing).
- Availability of construction materials and equipment.
- Reliability of data used to estimate wave conditions.

It appears that  $H_s$  for Dowden's Point breakwater was based on  $H_s$  200 year return and appears it has been factored to account for other height parameters and sea level rise of 1.0m or storm surge. We consider this a valid design decision.

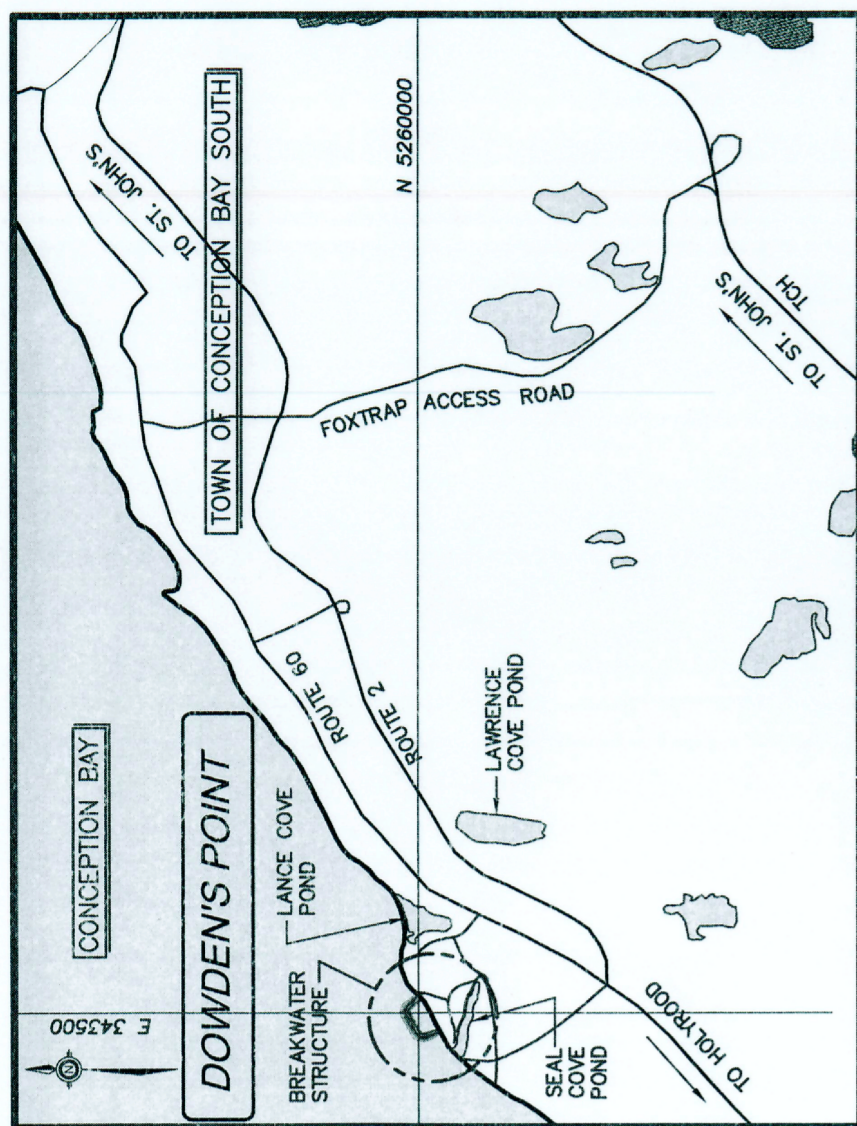


The risk of a wave damage to a rubble mound breakwater is considered less than the similar breakwater in L'Anse Au Diable given the sheltered conditions of Dowden's Point in Conception Bay.

Given the economic importance of this breakwater in protecting the HVDC electrodes, and the global trend of larger and more frequent storms the factor of  $H_{10}$ ,  $H_5$ , or  $H_1$  could be considered if the owner would like to increase the long-term reliability for the breakwater structure at Dowden's Point. We would recommend continual inspection and monitoring of the breakwater to validate the current design and construction and whether the above consideration is required.

# **Appendix A**

## **Design Drawings**



**LOCATION PLAN**

**NOTES:**

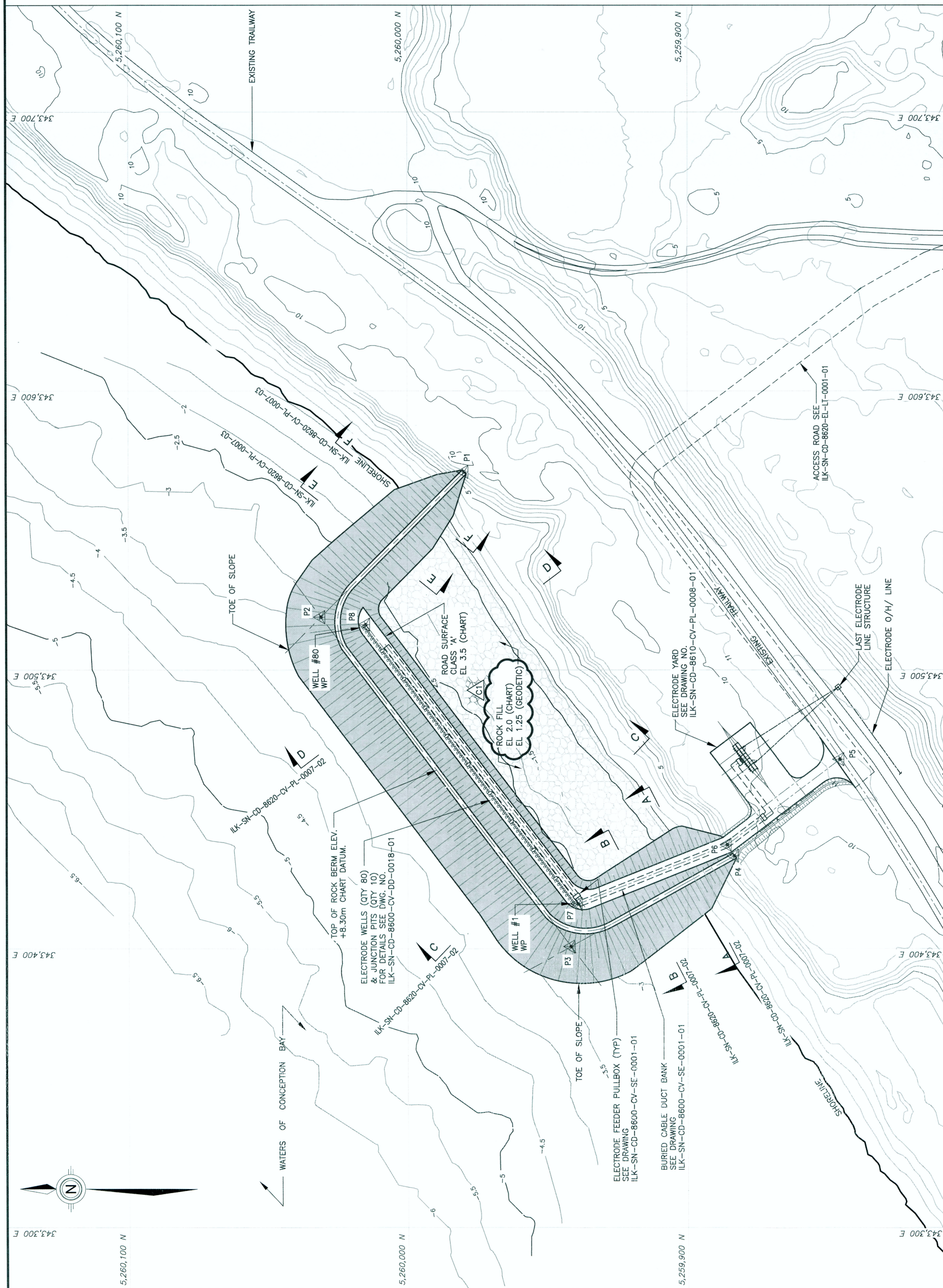
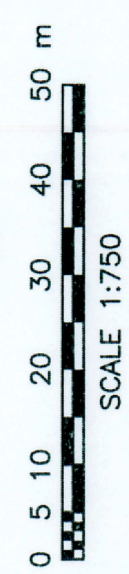
1. PROJECTION NAD 83 (CSRS) UTM ZONE 22
2. ALL BREAKWATER ELEVATIONS ARE IN CHART DATUM.
3. 0.0m GEODETIC = 0.75m CHART ELEVATION.
4. CHART DATUM ELEVATION REFERENCED FROM CANADIAN HYDROGRAPHIC SERVICE BENCHMARK 02F9150 & 62F9503.

**LEGEND:**

- TOPOGRAPHIC CONTOUR (GEODETIC)
- BATHYMETRIC CONTOUR (CHART)
- EMBANKMENT SLOPE
- BURIED CABLE DUCT BANKS
- ROCK FILL

POINT NO.	NORTHING	EASTING
P1	5,259,979.4	343,571.0
P2	5,260,031.1	343,519.0
P3	5,259,941.7	343,400.8
P4	5,259,883.3	343,433.3
P5	5,259,845.4	343,468.0
P6	5,259,885.9	343,437.3
P7	5,259,939.2	343,416.0
P8	5,260,015.0	343,516.3
WELL #1 WP	5,259,941.1	343,416.1
WELL #80 WP	5,260,016.2	343,515.4

PROVINCE OF NEWFOUNDLAND  
PERMIT HOLDER  
CLASS "A"  
This Permit Allows  
SNC-LAVALIN Inc.  
To practice Professional Engineering  
Permit No. as issued by PEGNL N0458  
which is valid for the year 2015.



**BREAKWATER PLAN**  
SCALE: 1:750

FOR INTERNAL USE ONLY

REVISION	DATE	BY	APP.	REVISION REGISTER
C1	25-FEB-2015	ROCKFILL EL. REVISED, LOCATION POINTS REVISED		
B3	11-JUN-2014	DESIGN OPTIMIZATION - GENERAL REVISION		
B2	19-MAR-2014	RE-ISSUED FOR BD		
B1	18-JUN-2014	ISSUED FOR BD		

REVIEW CLASS	EQUIPMENT TAG NUMBER	REVISION REGISTER
1. REVIEWED AND ACCEPTED NO COMMENTS		
2. REVIEWED - INCORPORATE COMMENTS, REVISE & RESUBMIT		
3. REVIEWED - INCORPORATE COMMENTS, REVISE & RESUBMIT		
4. INFORMATION ONLY		
5. NOT REVIEWED		

NO.	DATE	REVISION	REVISION REGISTER
1	11-JUN-2014	DESIGN OPTIMIZATION - GENERAL REVISION	
2	19-MAR-2014	RE-ISSUED FOR BD	
3	18-JUN-2014	ISSUED FOR BD	

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3	18-JUN-2014	ISSUED FOR BD	

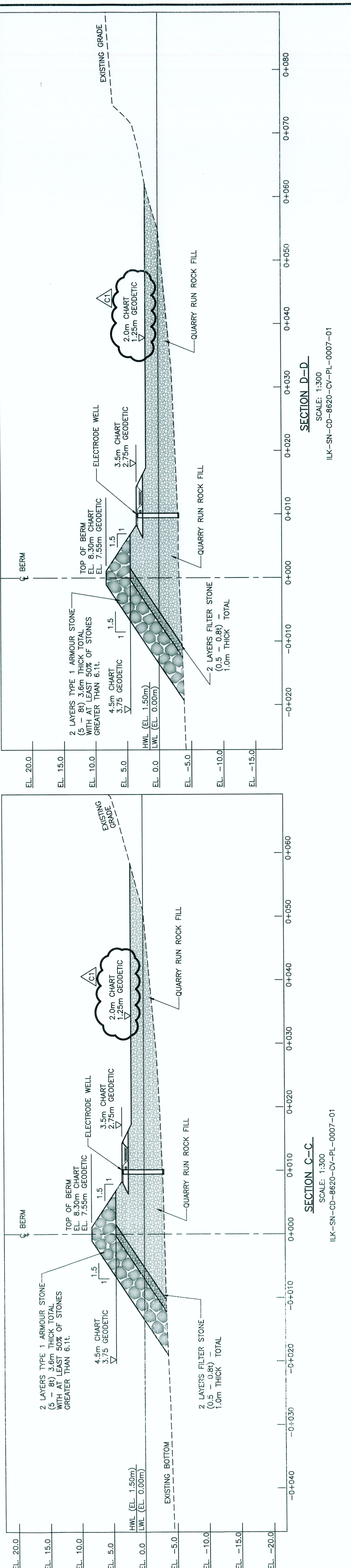
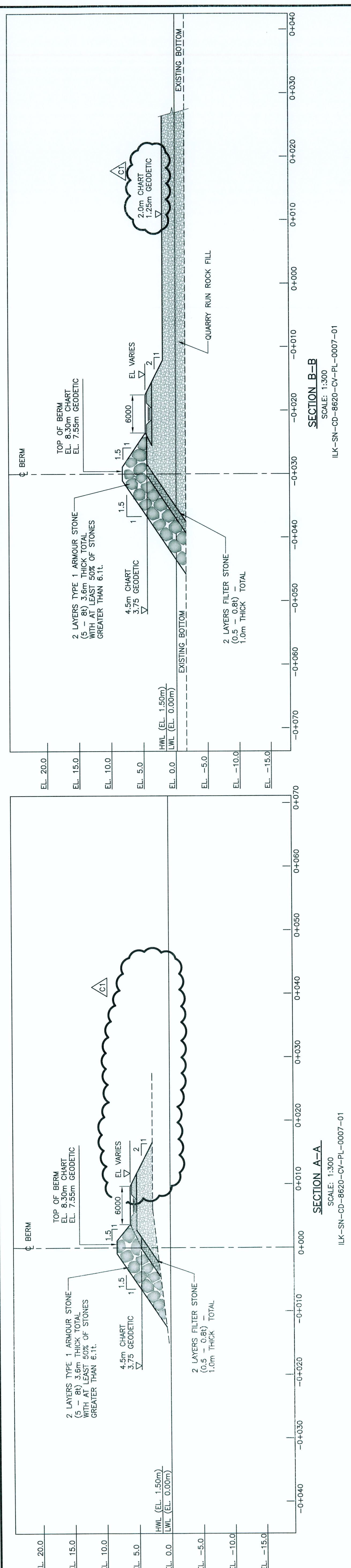
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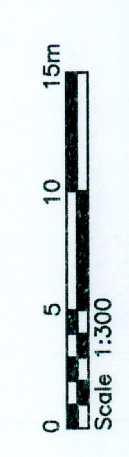
PROFESSIONAL STAMP: PEGNL N0458, CHRISTOPHER M. TUDGE, 2015-02-27, NEWFOUNDLAND, S. B.

CLIENT: SNC-LAVALIN  
PROJECT: LOWER CHURCHILL PROJECT  
TITLE: DOWDEN'S POINT ELECTRODE STATION BREAKWATER PLAN  
DESIGNED: N. GILLIS  
DRAWN: B. MURRAY/CMC  
VERIFIED: C. FUDGE  
DATE: 02-OCT-2013  
SCALE: AS NOTED  
SIT DOC NO.: 505573-8620-41DD-2002-SH1\_03  
NE DOC NO.: ILK-SN-CD-8620-CV-PL-0007-01  
Rev. C1



- NOTES:**
1. PROJECTION NAD 83 (CSRS) UTM ZONE 22
  2. ALL BREAKWATER ELEVATIONS ARE IN CHART DATUM.
  3. 0.0m GEODETIC = 0.75m CHART ELEVATION.

**PROVINCE OF NEWFOUNDLAND**  
**PIEBA PERMIT HOLDER CLASS "A"**  
 This Permit Allows  
**SNC-LAVALIN Inc.**  
 To practice Professional Engineering in Newfoundland and Labrador, NO.658 which is valid for the year 2015.



<p>REVIEW CLASS: FOR INTERNAL USE ONLY</p> <p>REVIEW DOES NOT CONSTITUTE APPROVAL OF DESIGN DETAILS, CALCULATIONS, TEST METHODS OR MATERIAL DEVELOPED AND/OR FROM FULL COMPLIANCE WITH CONTRACTUAL OR OTHER OBLIGATIONS.</p> <p>1. REVIEWED AND ACCEPTED - NO COMMENTS</p> <p>2. REVIEWED - INCORPORATE COMMENTS, REVISE &amp; RESUBMIT</p> <p>3. REVIEWED - INFORMATION ONLY</p> <p>4. NOT REVIEWED</p> <p>5. NOT REVIEWED</p> <p>DATE (dd-mm-yyyy) 27-FEB-2015</p> <p>DATE (dd-mm-yyyy) 27-FEB-2015</p> <p>DATE (dd-mm-yyyy) 27-FEB-2015</p> <p>DATE (dd-mm-yyyy) 27-FEB-2015</p>		<p>PROFESSIONAL STAMP</p> <p><b>SNC-LAVALIN</b></p> <p>DESIGNED: N. GILLIS</p> <p>DRAWN: B. MURRAY / C. LAING</p> <p>VERIFIED: C. FUDGE</p> <p>DATE: 03-OCT-2013</p> <p>SCALE: AS NOTED</p> <p>SU DOC No.: 505573-8620-41DD-2002-SH2-02</p> <p>NE DOC No.: ILK-SN-CD-8620-CV-PL-0007-02</p> <p>Rev: C1</p>																																
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